## STATE OF NEW YORK DEPARTMENT OF TRANSPORTATION

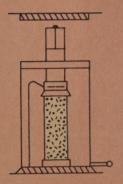


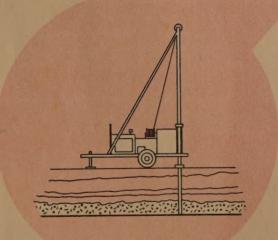
# SOIL MECHANICS BUREAU

CASE STUDY FEBRUARY, 1992









SUBJECT: 36 INCH CAISSON INSTALLATION

AT BASCULE PIERS

PROJECT: GREENPOINT AVENUE BRIDGE

OVER NEWTOWN CREEK

KINGS AND QUEENS COUNTIES

PIN 0751.65.301

CONTRACT NO. D500026



### NEW YORK STATE DEPARTMENT OF TRANSPORTATION SOIL MECHANICS BUREAU

36 INCH DIAMETER CAISSON INSTALLATION CASE HISTORY

PIN 0751.65.301
GREENPOINT AVENUE BRIDGE
OVER NEWTOWN CREEK
KINGS AND QUEENS COUNTIES

BY
ROBERT G. STROHMAIER
ASSISTANT SOILS ENGINEER
JANUARY, 1992

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#### ABSTRACT

This report describes the design and construction procedures used to construct the two bascule piers for the Greenpoint Avenue bridge over the Newtown Creek in New York City.

The bascule piers were supported on eighty-eight, 90 feet long, 36 inches in diameter steel caissons, with 1 inch thick walls.

Due to Maintenance and Protection of Traffic concerns, this Contract was phased and divided into five separate stages. The actual caisson installations for the bascules piers were performed by different contractors in Stage I and Stage IV.

Construction problems and recommendations related to the caisson installation procedure are discussed in this report.

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#### PROJECT DESCRIPTION

The project consists of a bridge replacement over Newtown Creek which separates the Boroughs of Queens and Brooklyn (Fig. 1). The total length of the structure is 1010 feet, and is supported on two standard abutments, seven hammerhead piers and two bascule piers. The abutments and hammerhead piers are supported on steel H-piles driven to practical refusal.

The bascule span is comprised of two equal sections 104 ft long for a total length of 208 ft. Each bascule section weighs approximately 375 tons and is designed to be raised and lowered by mechanical means employing counterweights and four 100 horsepower motors. Each bascule pier is supported on 36 inch diameter by 1 inch thick by 90 feet long, steel caissons seated at least six inches into rock. Thirty inch rock sockets were installed a minimum of four feet into sound rock. A total of 88 caissons were installed for these piers. A 14 foot thick tremie seal was necessary at both bascule locations to counteract the hydrostatic head developed at the proposed footing elevations.

This structure was constructed in five stages (Stage I through Stage V) primarily as a means to maintain traffic flow. In Stage I, the south section of the new structure was completed while traffic was maintained on the existing structure. In Stage IV, the original structure was then demolished while traffic was maintained on the south section of the new structure. The north side of the structure was then constructed. All caissons used for this structure were installed in Stage I and IV.

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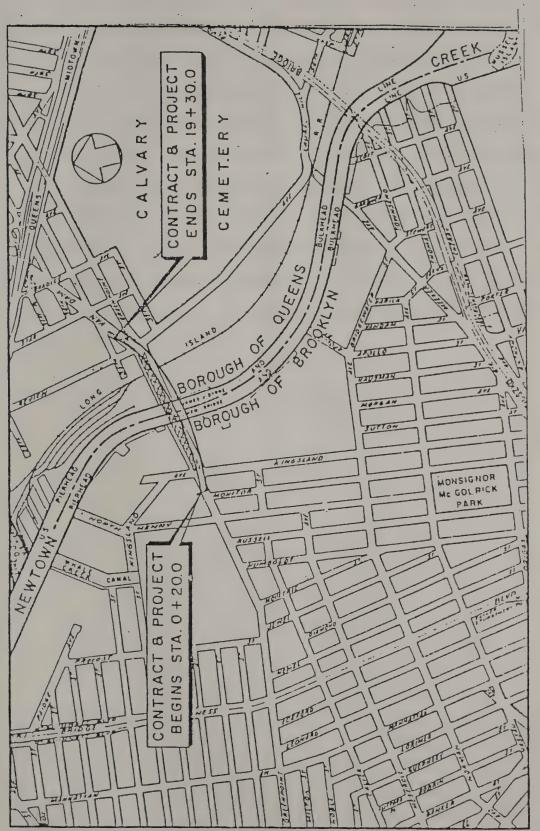
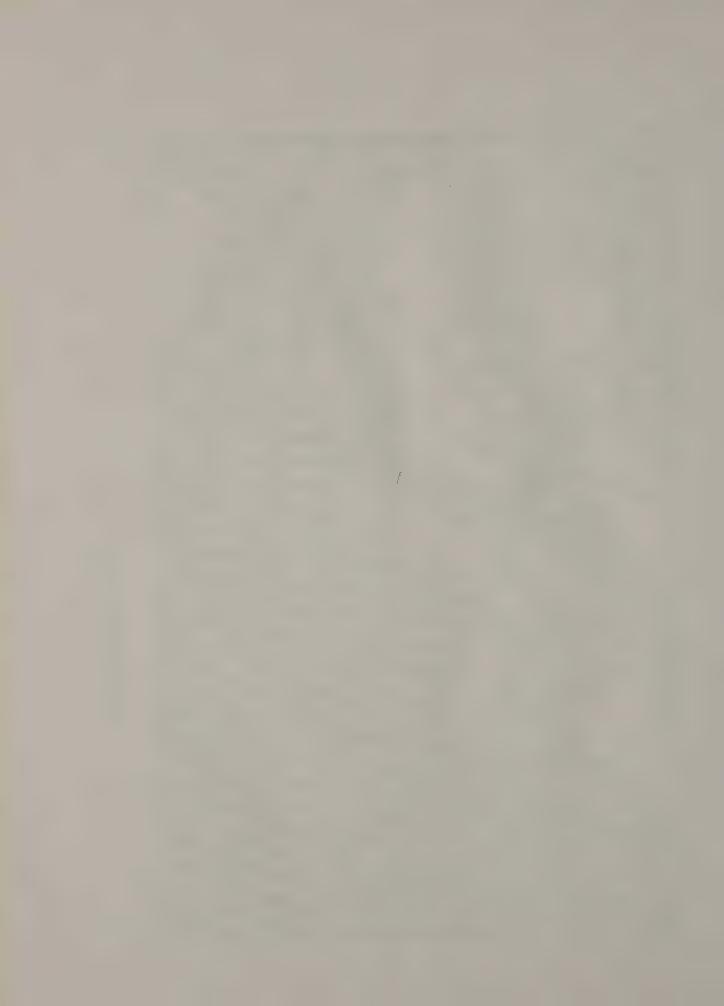
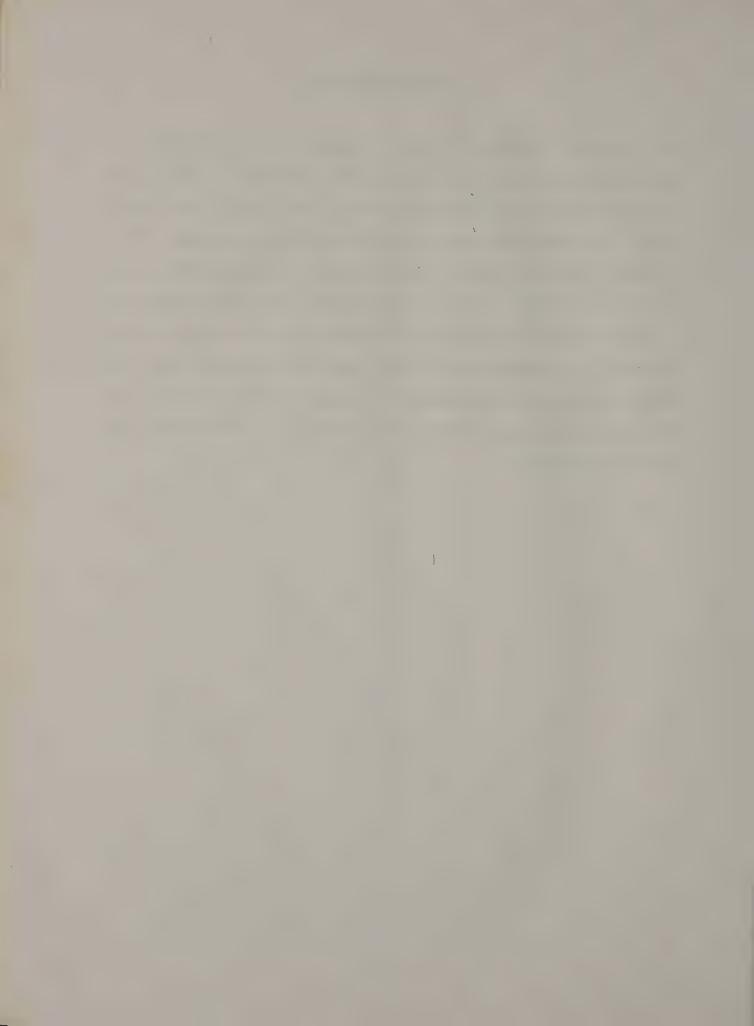


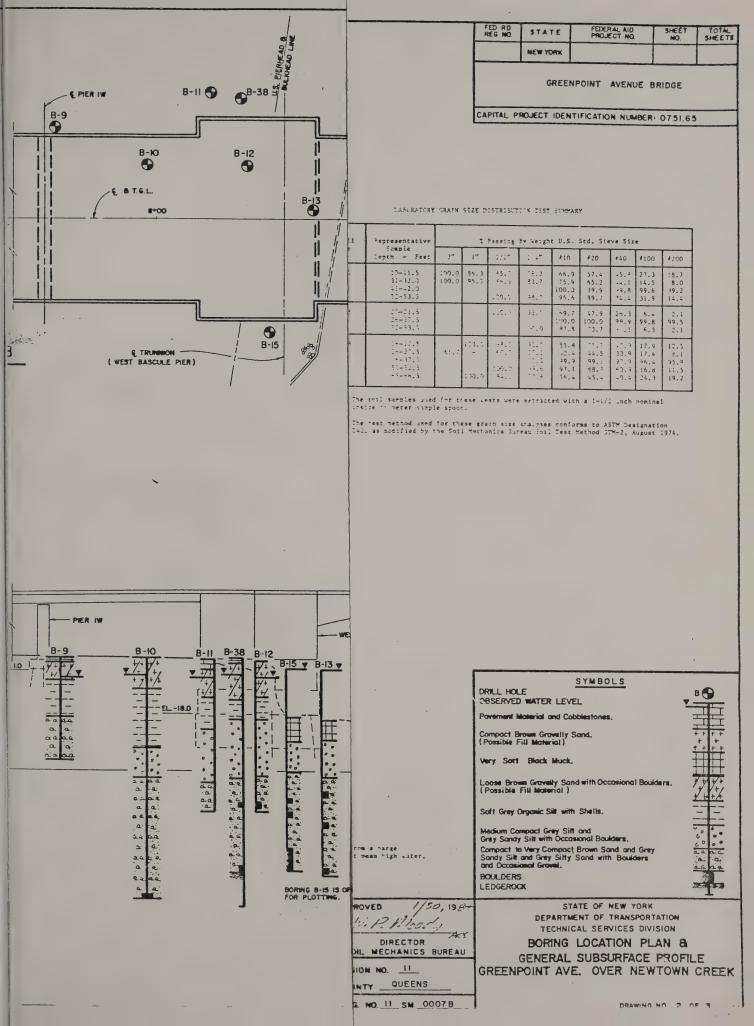
FIG 1



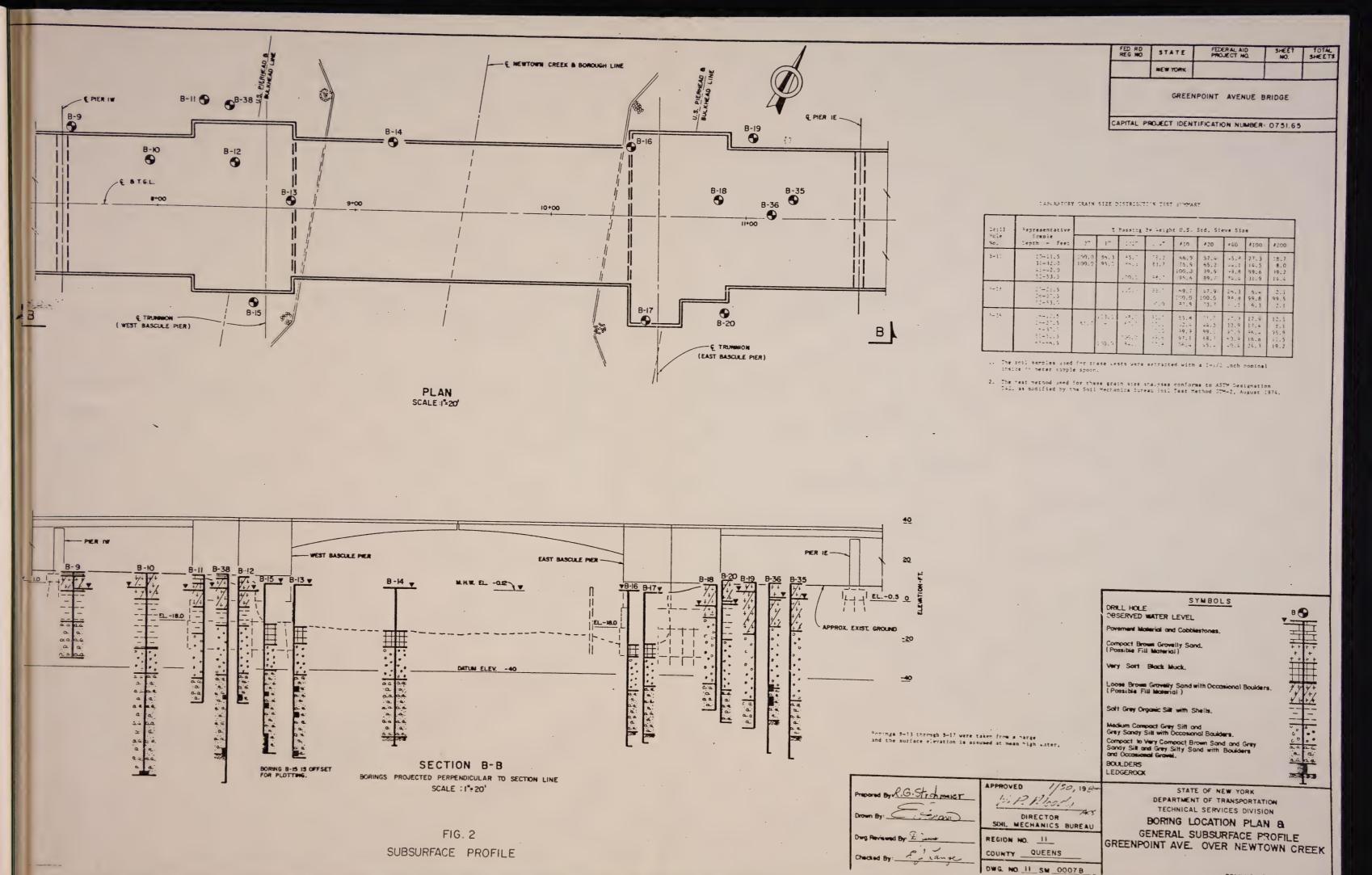
#### FOUNDATION CONDITIONS

The subsurface conditions are shown in Figure 2. In general, the site conditions in the area of the bascule piers consisted of a 12 to 16 foot layer of fill material described as Compact Brown Gravelly SAND overlying a 7 to 9 foot thick layer of Very Soft Black MUCK, changing to 10 to 15 feet of Soft Grey Organic SILT and Shells. Underlaying this was 40 to 50 feet of Medium Compact to Very Compact Brown Sandy SILT, Gravelly with occasional Boulders underlain by Ledgerock which in this area was described as an unweathered gneiss. Engineering Geologists from the Soil Mechanics Bureau recommended an allowable bearing capacity of 60 TSF in the rock, and an allowable side resistance of 6 TSF for each foot of rock socket depth.

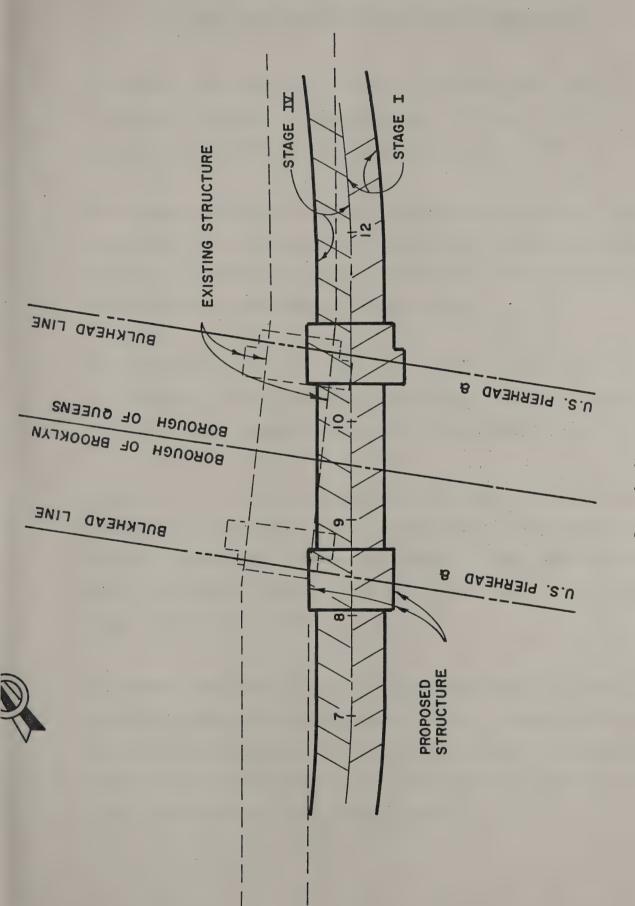












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#### CAISSON INSTALLATION PROCEDURE (STAGE I CONSTRUCTION)

The bascule pier constructed in Stage I required that a total of 53 caissons be installed. Construction began in June of 1985, and was completed several months behind schedule in November of 1986.

The original specification provided for an installation method of drilling ahead of the cutting shoe for the full depth of the caisson until the shoe was on bedrock. It also specified that a net positive head be maintained in the shell using a bentonite slurry.

No driving of the shell was to be allowed until the original structure was demolished. There were some concerns about the adverse effect of vibrations on the original structure while it was still in use.

The prime Contractor, Morrison-Knudsen-Yonkers (MKY), requested permission to use an alternate installation method which was approved by the Structures Design and Construction Division. This method involved driving the caisson shells with an MKT Diesel hammer having a rated energy of 100,000 ft. pounds.

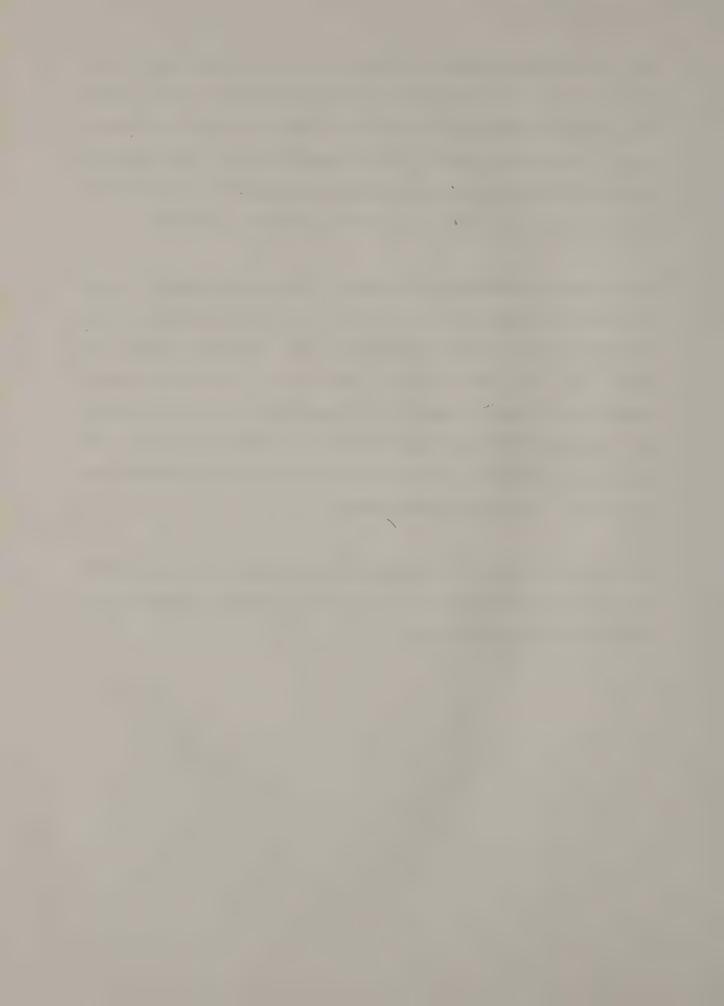
The caisson shells were driven to a maximum resistance of 5 blows/foot and cleaned using a three step operation. First, a chisel beam was used to loosen and break up the material. Second, an air lift was inserted to remove the loosened material. Finally a Casagrande grab bucket was used to accomplish the final cleaning operation.



MKY experienced difficulty driving the shells through undisturbed material. Several casings were damaged during driving. Three casings were so badly damaged that an internal inspection had to be performed using a video camera lowered into the damaged casings. One caisson was rejected and another was installed as close as possible to the rejected one. The other two caissons were judged structurally acceptable.

This method of installation also made it difficult to completely clean the shell out before driving was resumed. To eliminate these problems MKY proposed a method of installation using a vibratory hammer (ICE Model 812). The State would not allow use of the vibratory hammer unless the Contractor installed instrumentation to monitor and ensure the integrity of the existing structure, at no cost to the State. The Contractor chose not to install the instrumentation and therefore was not allowed to use the vibratory hammer.

The rock sockets for these caissons were progressed in the same manner for both Stage I and Stage IV. A detailed discussion is included in the Stage IV installation procedure.



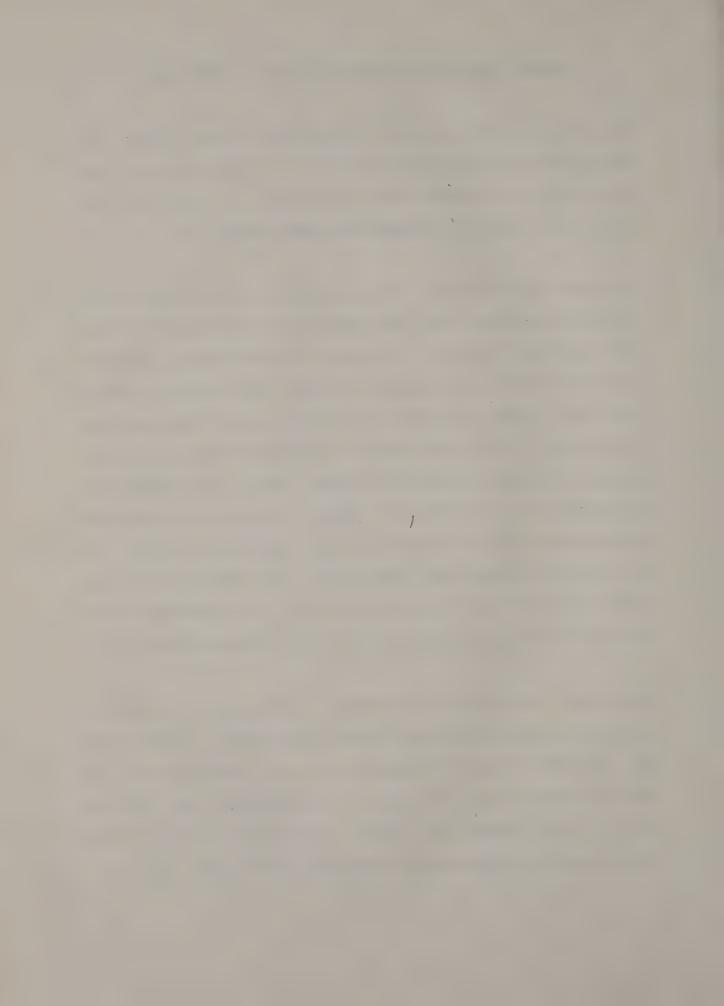
#### CAISSON INSTALLATION PROCEDURE (STAGE IV CONSTRUCTION)

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Stage IV construction consisted of completing the south section of the new structure. For Stage IV construction, the prime Contractor (MKY) sub-contracted the bascule caisson installation to Franki Pile Foundations. They used the following installation method.

The caisson shells were set into place using a 100 ton crawler crane. All shells were driven with a 7000 pound caisson drop hammer with a two foot drop (Photo 1 and 2). A criterion of 5 blows/inch was established as a guideline for maximum penetration without overstressing the shells. When this set was achieved, the overburden inside the casing was loosened with a 42 ton churn drill. The churn drill bits were 33 inch diameter star drill configurations (Photo 5 and 6). The drilling was progressed 3 to 5 feet below the cutting shoe and the cuttings were removed with a bailer or sand pump to the tip elevation (Photos 7 and 8). Driving operations were then resumed. This process was continued until the caisson was seated on bedrock. The approximate caisson lengths were 80± feet on the west side and 60± feet on the east side.

Rock socket installations were identical in this stage as in Stage I. After seating the cutting shoe attached to the caisson, a pilot boring was progressed so that the Engineering Geologist could evaluate each rock core and determine the depth of the rock socket. Once this was done, 30 inch diameter rock sockets were drilled with 42 ton churn drills using drill bits weighing approximately 7000 pounds. (Photo 9).

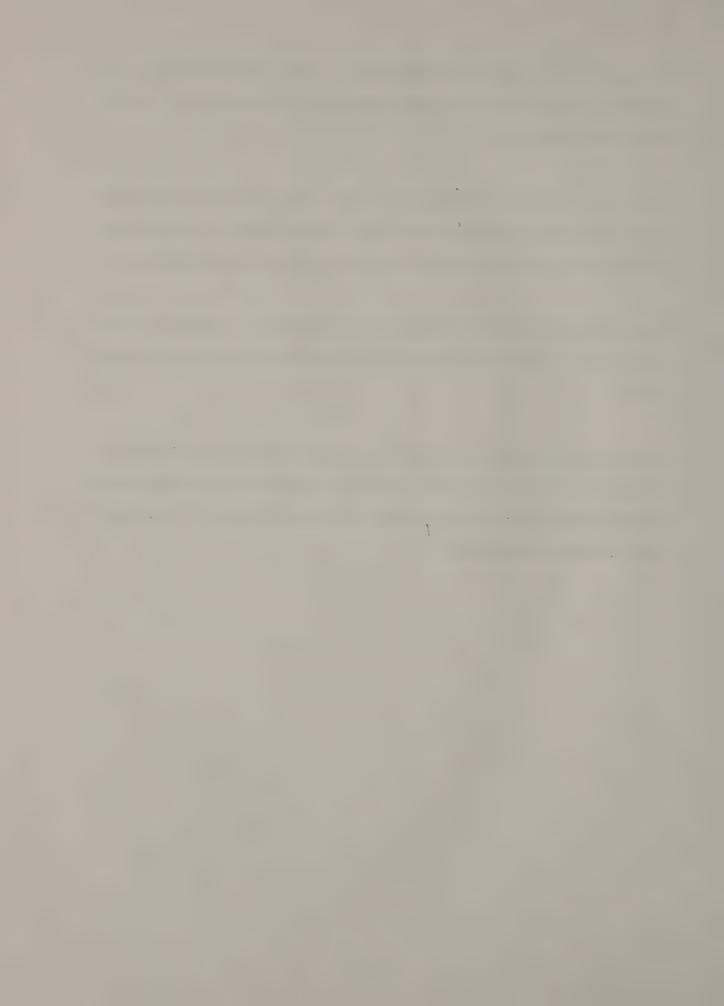


The steel rebar cages were fabricated at the construction site and lowered to the bottom of the rock socket and centered in the caisson. (See Photos 3 and 4).

There were 35 caissons installed in this stage and 14 were inspected using video equipment for the full length of the caisson. Based on these inspections all 35 caissons were judged to be acceptable installations.

These inspections were to ensure proper cleaning and seating of each caisson and to determine that no major collapsing of the shell had taken place.

The EIC informed this writer that no major problems were encountered during this stage. This is due primarily to the Contractor's decision to drill ahead of the caisson shells thus enabling it to be advanced using a lower driving energy.



#### CONCLUSIONS

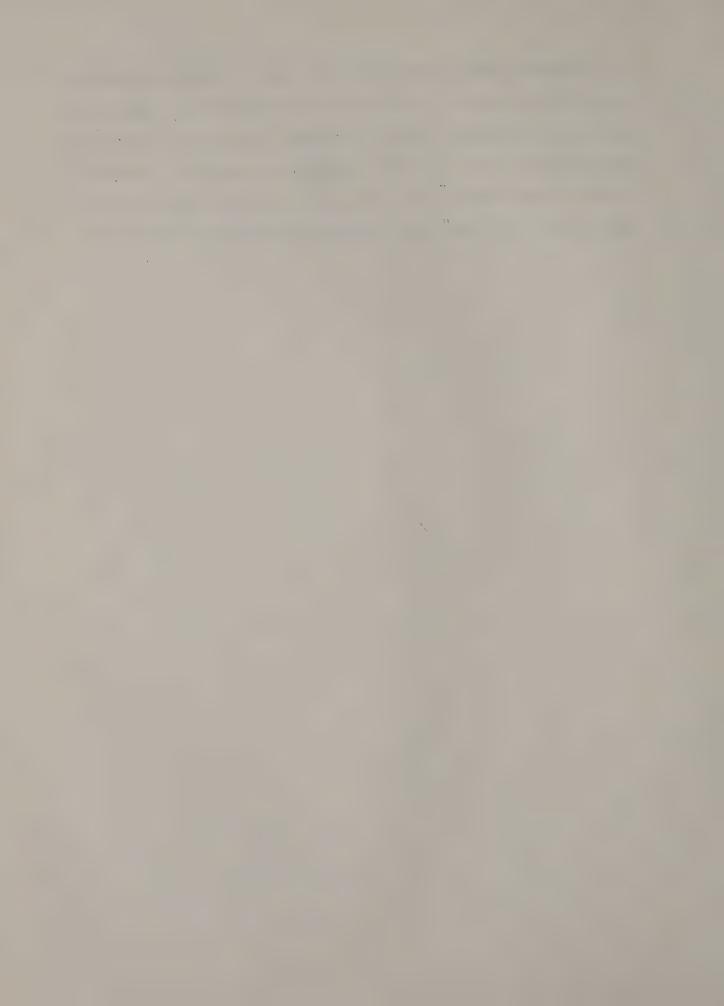
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Several conclusions can be drawn from the Stage I and Stage IV installations:

- 1. The most evident conclusion drawn from this project is that the Stage IV caisson installation was a more efficient and smoother operation than the methods used in Stage I. Franki Pile Foundation's knowledge and experience with caisson construction and the use of proper equipment were quite evident.
- 2. A prequalification requirement should be included in the specification so that a subcontractor who is familiar with caisson installation procedures, such as Franki Pile Foundations, will perform the required work. This should insure that an acceptable product will be provided prior to any work commencing and would eliminate construction slow downs caused by alternate construction proposals advanced by inexperienced contractors.
- 3. Since the caissons were designed for end bearing only, our specification requiring drilling ahead of the caisson during installation in compact or bouldery soils to prevent damage to the shells should not have been compromised.
- 4. On this project, or any similar project where stage construction is necessary, the specification should preclude the use of vibratory equipment where any existing structure or building may



be in jeopardy without incorporating some type of vibration monitoring program in the contract. On this project, MKY requested our approval to use vibratory equipment and the State had to prepare a monitoring program during construction which ultimately was abandoned. Preparation of this program required extra time and cost to the State and an unneeded delay to the Contractor. (See Stage 1 Construction Procedure).



DESCRIPTION. The work shall consist of furnishing and installing drilled, steel shelled, concrete filled caissons at the locations indicated on the Plans.

MATERIALS. Materials used for this work shall conform to the following requirements:

Steel. Steel shells shall meet the requirements of subsection 715-01, ASTM Designation A252 - Grade 2. Shells shall be seamless. All shells shall be equipped with a cutting shoe acceptable to the D.C.E.S.

Reinforcing Steel. This shall meet the requirements of subsection 709-04, Epoxy-Coated Bar Reinforcement.

Concrete. The requirements of Section 501 shall apply.

Concrete shall be Class G.

#### CONSTRUCTION DETAILS.

Steel Shell Installation. Equipment used for this work shall be the responsibility of the Contractor.

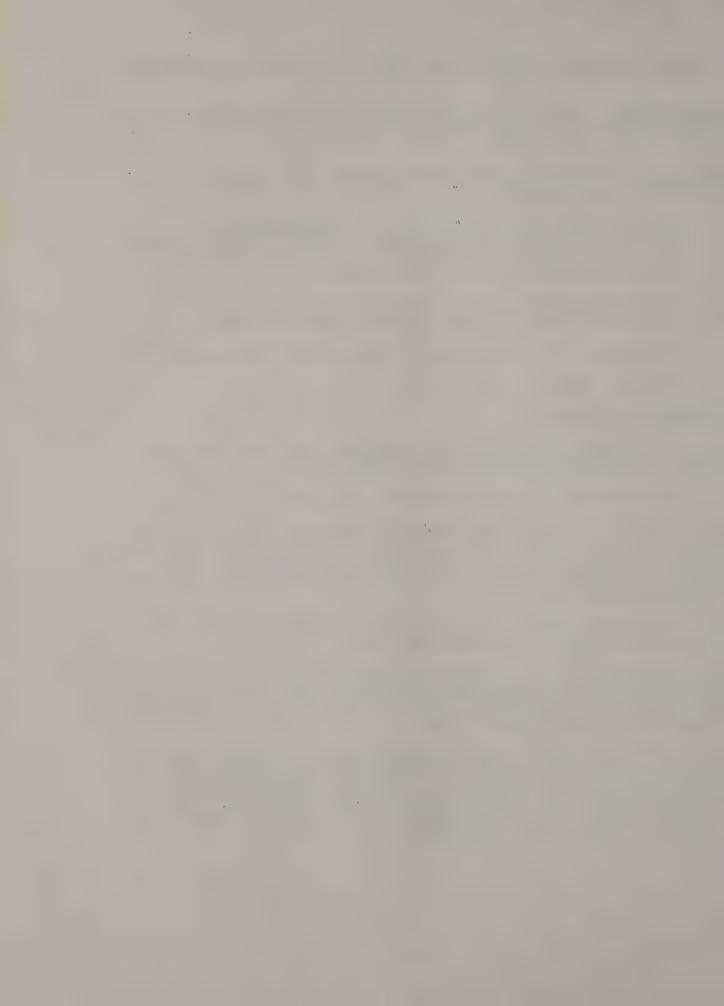
The method of installation shall be as follows:

Drill, or auger, in advance of each shell as it is being progressed, to remove all soil and other obstructions from the proposed path, until the cutting shoe is in contact with bedrock. A net positive head shall be maintained at all times using drilling mud.

Driving of the steel shells will be permitted after the existing structure has been demolished.

Splices will be allowed only if specifically permitted by the D.C.E.S. and at locations approved by him. Only one splice per pile will be allowed. Splices shall be made in the manner detailed on the Contract Plans.

After the shell has been progressed to the point that the cutting shoe is in contact with bedrock, all materials inside will be completely cleaned out. The cutting edge shall then be seated firmly into sound rock for a depth of at least six (6) inches. After the cutting edge has been seated, a rock socket shall be drilled into the bedrock.



Drilled-in rock sockets shall be a minimum of 30 inches in diameter and shall extend to a depth of at least four (4) feet below the cutting shoe.

The depth of the rock sockets will be based upon pilot borings extending a minimum of ten feet into sound rock. Borings will be taken at the locations indicated on the Plans. The actual depth of each rock socket will be determined by the D.C.E.S. or his designated representative. The D.C.E.S. will render this determination five working days after the receipt of all pertinent information. The Contractor's attention is directed to the Special Notes which pertain to pilot borings which shall be drilled in accordance with the requirements for Item No. 648.22 - Rock Core Drilling NX Cores 2 1/8" Minimum Diameter.

After the rock socket has been drilled, socket and shell shall be thoroughly cleaned and inspected. Any evidence of misalignment greater than three inches from the Plan location or of distortion shall be reported to the D.C.E.S. who will make a determination as to the corrective measures, if any, to be taken. All corrective measures ordered by the D.C.E.S. will be done at no additional expense to the State.

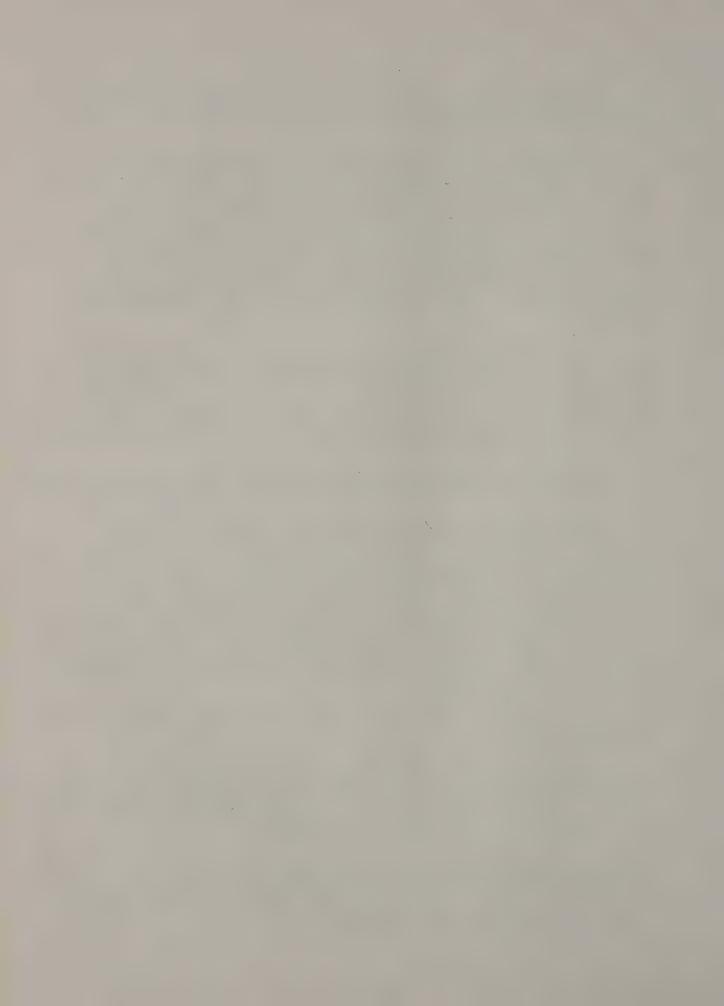
Bar reinforcement shall then be placed and secured such that it will occupy its proper position within the concrete fill.

After obtaining approval from the Engineer concrete placement may begin. Placement shall be such that segregation of the mix is avoided. Under no circumstances will concrete be permitted to be dumped freely into the shell. Special care shall be exercised during placement to prevent honeycombs and air pockets from forming. Precise volumetric records shall be kept for the filling of each shell to verify the proper placement of concrete. Internal vibrators, and other means acceptable to the Engineer, shall be used to consolidate the concrete. Internal vibrators, and other means, shall be used to the depth deemed practical by the Engineer.

Concrete shall be placed in accordance with the requirements of Section 555.

Completely installed caissons shall vary not more than 2% from the vertical or from the angle of batter indicated on the Plans. They shall not vary from the plan location by more than three (3) inches in any direction. Should this provision be violated the D.C.E.S. shall be informed.

METHOD OF MEASUREMENT. Measurement will be made as the number of linear feet of steel shell drilled caissons installed. Measurement will be taken along the centerline of the caissons from the top of cut-off elevation to the bottom of rock socket, before the concrete fill is placed.

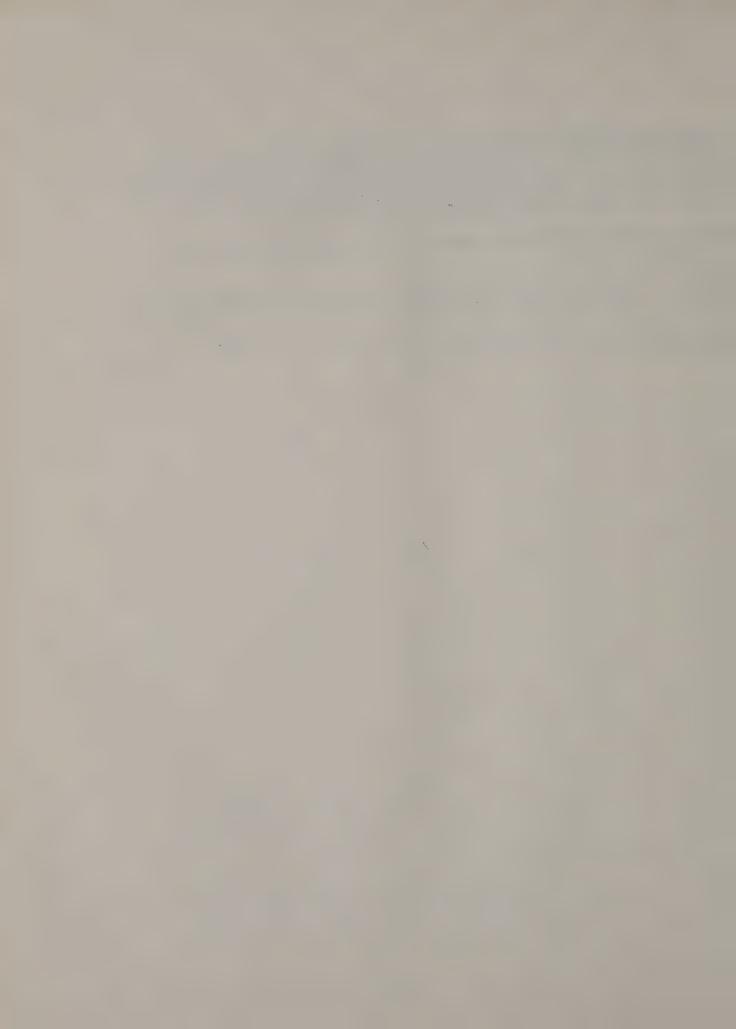


BASIS OF PAYMENT. The unit price bid per linear foot shall include the cost of all labor, materials and equipment necessary to complete the work including splicing, and cutting off surplus shell lengths. No payment will be authorized until after the shell has been filled completely with concrete.

Epoxy-Coated Bar Reinforcement will be paid for under its respective item.

Pilot borings for rock sockets will be paid for under Item No. 648.22 - Rock Core Drilling NX Cores 2 1/8" Minimum Diameter.

No payment will be made for work specifically excluded from payment by the terms of this item.



1648-1 DESCRIPTION. This work shall consist of drilling for soil and rock samples and recording boring data as specified. The various sampling methods are designated as follows:

Split Barrel Sampling — Driven soil samples Thin-Walled Tube Sampling — Undisturbed soil samples Rock Core Sampling

648-2 MATERIALS. Materials for this work shall meet the requirements of the following subsections of Boulder and Rock Core Boxes 732-10

## 648-3 CONSTRUCTION DETAILS

648-3.01 Drilling Procedures.

B. Rock Samples. Continuous rock core samples shall be taken in boulders exceeding six inches in penetration thickness and in ledge rock at locations and depths designated by the Engineer. Rock samples shall be drilled with a Double Tube, Swivel Type Core Barrel using a diamond bit that will drill a rock core not less than 1 1/8 inches in diameter in 2 1/2 inch casing and 2 1/8 inches in diameter for 4 inch casing. If, in the judgement of the Engineer, a satisfactory core is not recovered, a Series M Double Tube Core Barrel or approved equal and short "runs" may be required.

648-3.02 Sampling.

C. Rock Cores. Core recoveries of less than 85% will generally not be considered satisfactory. Except for deliberately serving the continuous core, care shall be taken to avoid blocking the bit or core barrel. When the core is broken off, the core barrel shall be raised to the ground surface and the core withdrawn.

648-3.03 Boring Log Data. Data obtained in borings shall be recorded in the field and shall include the following:

C. nock Samples. Samples shall be labeled in accordance with "Instructions for Labeling Rock Cores" as shown on Soils Bureau Drawing No. SM1694. This information is available upon request to the Department's Soil Mechanics Bureau.

The rock cores shall be placed for shipment in "Boulder and Rock Core Boxes" meeting the requirements of §732-10.

648-3.04 Ground Water Levels. The level at which ground water is first encountered in the borings shall be noted. When water is introduced in the hole, water level readings shall be taken at the end of each day after the hole is prepared to the next sampling depth. The casing or drive pipe shall be filled with water and covered at the end of the working day and the drop, if any, recorded when the work is resumed. Ground water levels shall be measured before and after the casing or drive pipe is pulled. Each water level reading shall be recorded showing the date and time the reading is made, the depth of the drive pipe or casing, and the depth to water. Any loss or gain of water in the boring, except that caused by deliberately introducing water and/or inserting or removing tools shall be recorded. This record shall show the date and time the loss or gain is noted, the depth of the casing and the depth to water. If flowing, the height of artesian rise above the ground surface shall be recorded.

All water level readings and related data shall be recorded on the borings logs under "Remarks." If necessary, additional forms shall be used for recording ground water data.

Artesian heads shall be effectively and permanently sealed. This seal shall be satisfactory to the Engineer.

## 648-3.05 Marking and Packaging Samples.

Cores' as shown on Soils Bureau Drawing No. SM1694. This information is available upon request to the Department's Soil Mechanics Bureau.

The rock cores shall be placed for shipment in "Boulder and Rock Core Boxes" meeting the requirements of §732-10.

## 648-3.06 Delivery of Samples.

B. Rock Samples. Samples shall be delivered to the New York Regional Office which is delegated the responsibility and authority to execute the prescribed work.

648-3.07 Furnishing Equipment for Making Borings. The Contractor shall furnish the number of drill rigs stated in the proposal. The rigs shall be supplemented with the necessary auxiliaries, appurtenances, tools and other equipment. All equipment shall be acceptable to the Engineer.

## 648-4 METHOD OF MEASUREMENT

648-4.03 Rock Core Drilling. The quantities to be paid for shall be the actual number of linear feet drilled as measured in the presence of the Engineer or Inspector and as substantiated by core length obtained, property labeled, described and boxed.

648-4.04 Furnishing Equipment for Making Borings. The quantities to be paid for shall be the number of drill rigs specified in the proposal and for additional rigs ordered on the project by the Engineer. Payment will not be made for any drill rig that does not work at least 75 percent of the total working time computed from the date of actual commencement of the work to the final completion date.

648-5 BASIS OF PAYMENT



648-5.03 Rock Core Drilling. The unit price bid per linear foot shall include the cost of furnishing all property of the State. Furnishing boring equipment will be paid for separately.

648-5.04 Furnishing Equipment for Makings Borings. The unit price bid for each drill rig shall include the cost of furnishing all labor, materials and equipment necessary for transporting, erecting, maintaining, labor, including the manipulation of the boring equipment and materials in connection with making borings shall be included in the unit price bid for the respective sampling items.

Payment will be made under:

Item No.

Item

Pay Unit

648.22 648.23

Rock Core Drilling NX Cores 2 1/8" Min. Diam. Furnishing Equipment for Making Borings

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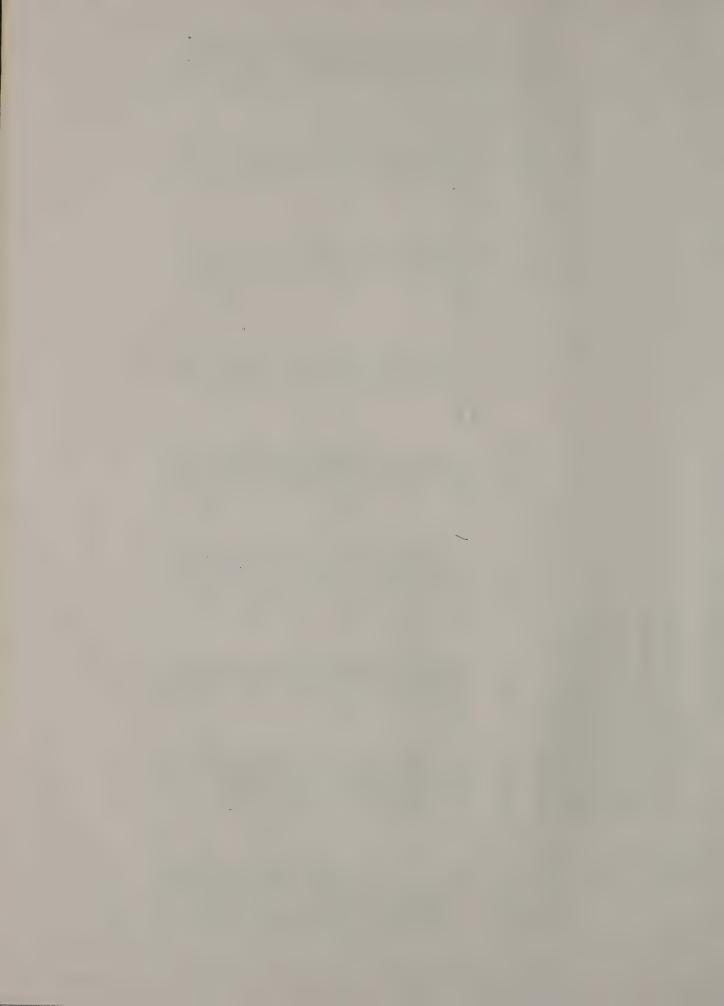
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	OUANTITY	-	17.00	62,400.00	38,750.00	8,582,00	1,588,00	61,796,00 B 969,00	69,593,00	52,300,00	2, 198, 274.00	23,662.00	3,110.00 100%LS	100%LS	2.00	28,00	28.00	2,00	100%15	286.00	73.00	73.00 2,797.00 2,485.00	
	Nati	- -	551,0803	-11551,1238 552.04	552,0601 552,0602 · 11552,95	553.03	555,0201	555.0403	655.0404 6555.06	16555,6602	558.0202	556,03	11560.98	564,0502	565, 1302.			565,1707	565.1716	566.2001	567.38	568,11 11569,1002	

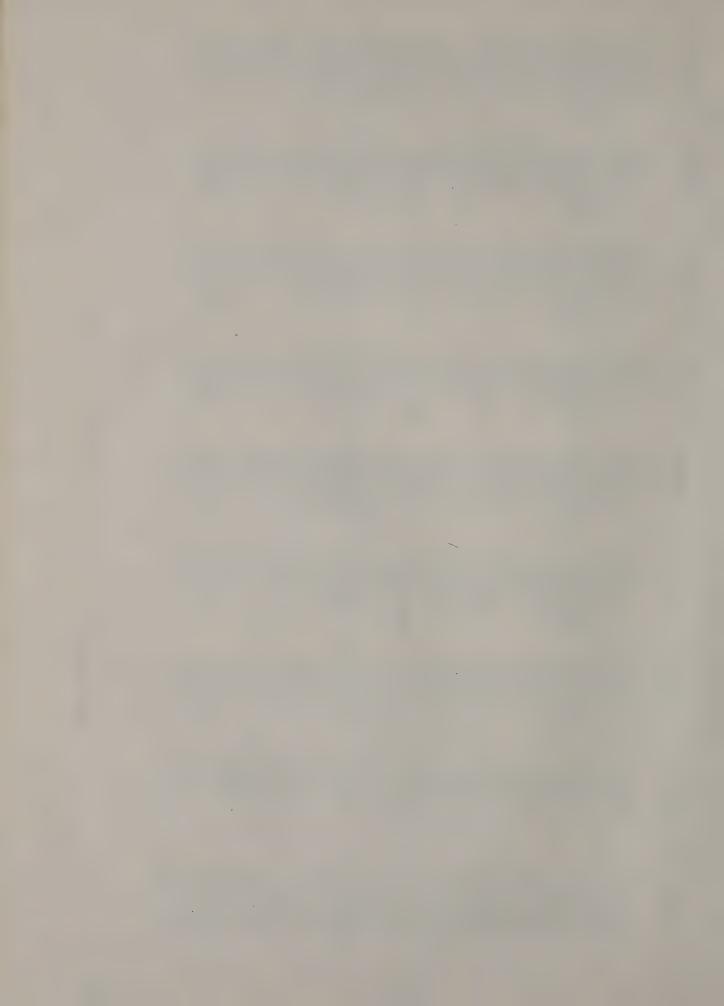


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NYS DEPARTMENT OF TRANSPORTATION
CONTRACTS BUREAU - SPECET - BID REPORT PROGRAM - J502000

LETTING DATE 8/30/84

CONTRACT NUMBER D500028

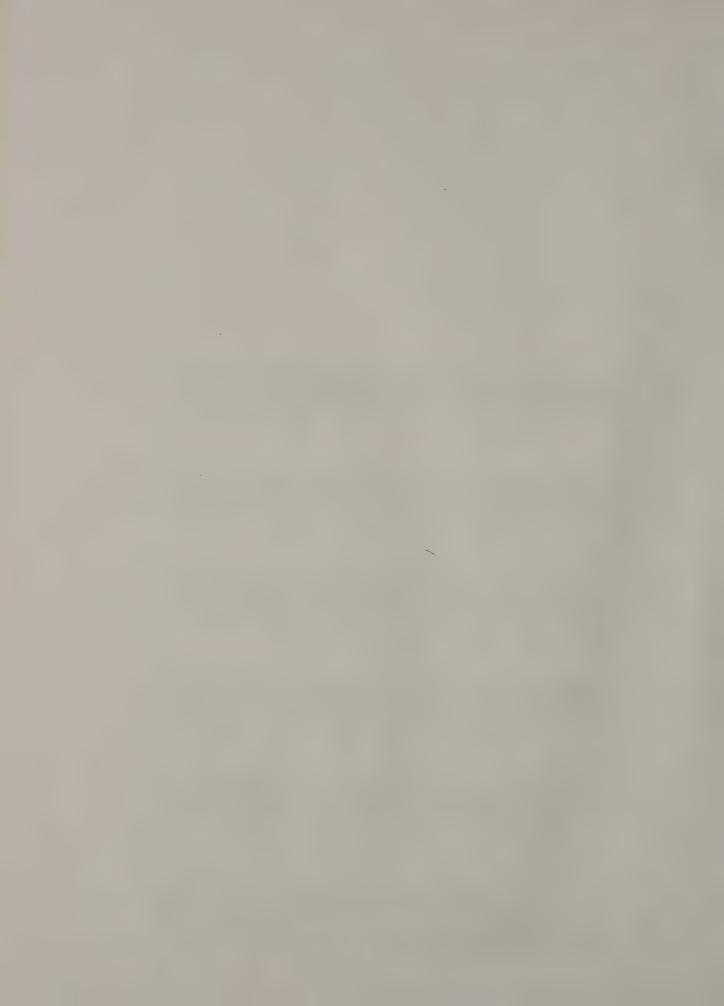


CONTRACTS BUREAU - SPREAD REPORT - BID REPORT PROGRAM - J502000 EEHS PIN 075165

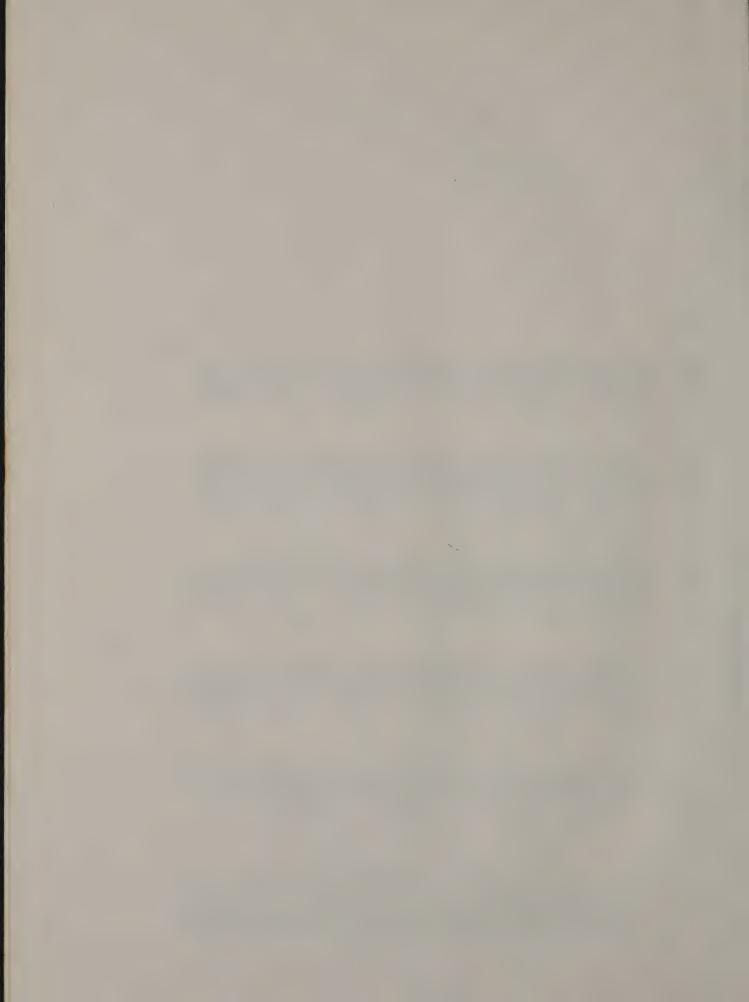
	BIDDER		270,000 23,000 610,000 3,400,000 2,000 1,500 100,000 1,160,000,000 1,160,000,000 39,574,264.25 39,574,264.00	
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BIDDER	20,000,000 15,000 15,000 15,000 21,000 21,000 21,000 21,000 21,000 21,000 21,000 21,000 10,000 10,000 10,000 100,0
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BIDDER	350,000 400,000 20,000 300,000 1,500,000 1,500,000 1,000,000	
BIDDER J	600.000 700.000 700.000 700.000 1,000.0000 1,000.0000 1,000.0000 1,000.0000 1,000.0000	
BIDDER	1,500.000 2,100.000 1,200.000 1,200.000 1,200.000 1,200.000 2,000.000 1,000.000	2000.000.
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QUANTITY	3,950.00  3,950.00  201.00  1,650.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,620.00  1,600.00  1,160.00	2.00
I EM		11663,2808



	CONTRACTS BUREAU - SPREAU REPORT - BID REPORT FROGRAM - JSG2000	
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	TING DATE 8/30/84 N V	
Name and Address of the Owner,	TING	

BIDDER

13,542,439.60 43,542,439.60

CONTRACT NUMBER DSOUG

		4	4	4
BIDDER	<b>5</b>	42,459,103.80	42,469,903.80	42,469,903,80
BIDDER		41,933,102.00	41,933,102.00	41,933,102,00
BIDDER	<b>=</b>	40,616,873,23 41,933,102.00 42,459,103.80	UM DF BIDDERS EXTENSIONS 40,633,857.50 41,933,102.00 42,469,903.80 DIFFERENCE 10,800.00	40,633,857,50 41,933,102,00 42,469,903.80
YTITAND,			EXTENSIONS	-
ITEM	•	OMPUTED TOTAL	UM OF BIDDERS DIFFERENCE	NNOUNCED BID

00.00

43,542,439.60

I HEREBY CERTIFY THAT THIS TABULATION INCLUDES ALL BIDS RECEIVED AT A PUBLIC LETTING HELD AT ALBANY, N.Y. ON B/30/84 FOR A CONTRACT IN KINGS [1600026

DATE

14/1/

14/84 3 Sentan 3 Stains





1 DROP HAMMER USED IN (STAGE IV) CAISSON INSTALLATION (SIDE VIEW)



2 END VIEW OF SAME HAMMER





3 JIG USED TO CONSTRUCT REBAR CAGE USED
IN CAISSONS



4 REBAR CAGE INSTALLED IN DRIVEN AND SEATED CAISSON





JIG MANUFACTURED BY FRANKI PILE (STAGE

IV) CONTRACTOR TO SHARPEN CHURN DRILL



6 ANOTHER VIEW OF SAME CHURN DRILL
SHARPENING EQUIPMENT





7 BAILER USED TO CLEAN OUT CASING (USED IN CLAY MATERIAL)



8 PIPES USED IN SAND PUMP CAISSON CLEANING
OPERATION





9 TRACK MOUNTED RIG USED FOR ROCK SOCKET DRILLING

